

HSS circular section in tension verification using S16-19:

1.1.1 Description

The test verifies the tension strength of circular HSS section column.

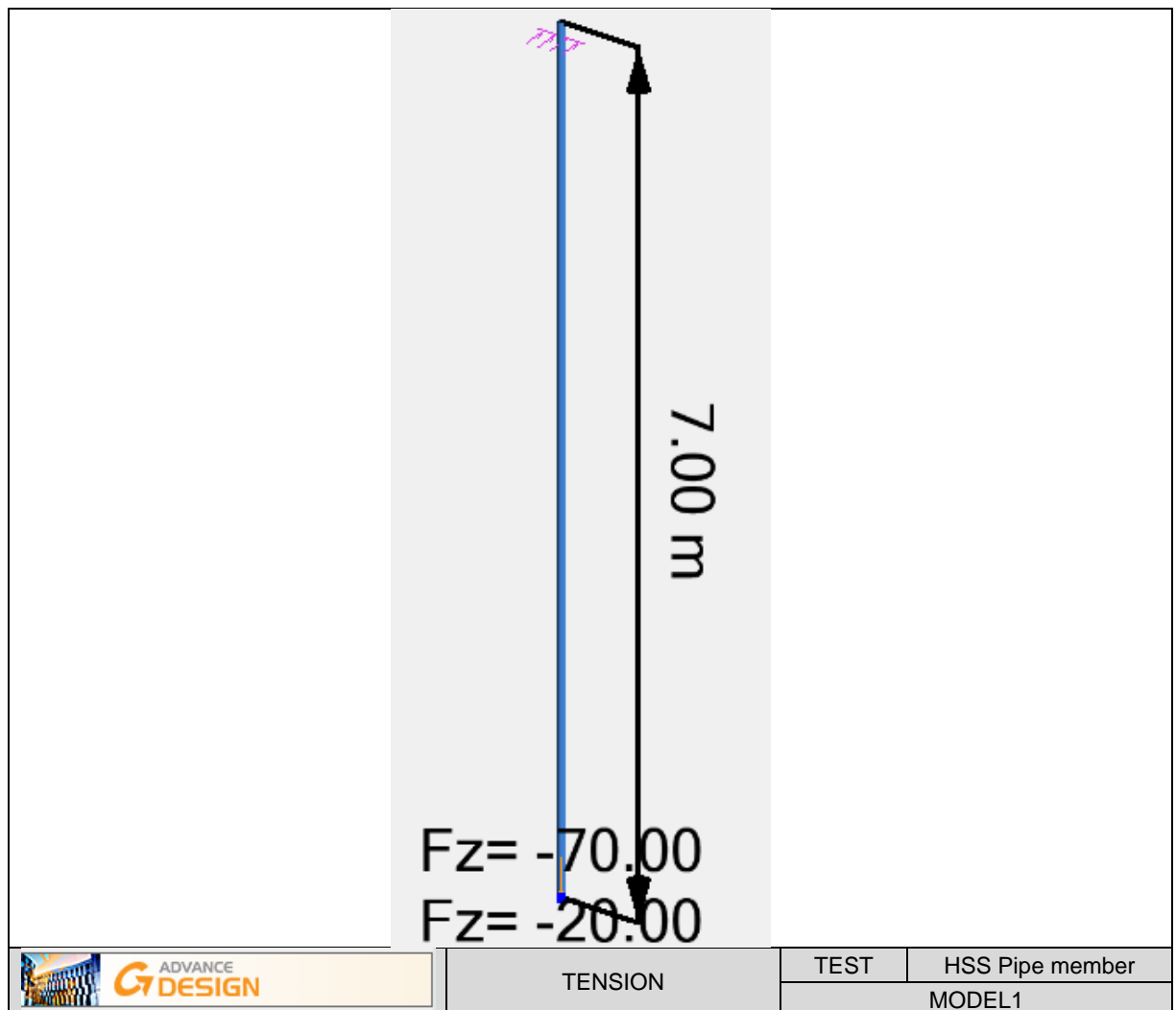
The column is subjected to end tension.

1.1.2 Background

The test verifies the available tension strength of an HSS pipe 76x4 column shown in the figure below. The column is subjected to tension of 20 kN from dead load and 70 kN from live load. Material G40.21M-350W ($F_y=350$ MPa) steel is selected for this example.

1.1.2.1 Model description

- Analysis type: static linear (plane problem)
- Element type: linear
- The following load case is used:
- Load cases: $F_D = -20$ kN : $F_L = -70$ kN

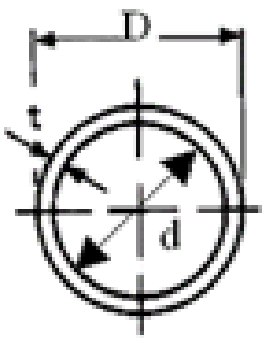


Units

Metric System

Geometry

- Cross section:

<u>Dimensions and surface area</u>		<u>Properties</u>		
	Area		Strong Axis (Y-Y)	
	A	8.19 cm²	I_y	54.10 cm⁴
	Diameter		S_y	15.46 cm³
	D	7.62 cm	Z_y	18.8 cm³
	Thickness		Weak Axis (Z-Z)	
	t	4 cm	I_z	54.10 cm⁴
			S_z	15.46 cm³
			Z_z	18.8 cm³
			Shear area	
			A_y	1.70 cm²
			A_z	1.70 cm²
			Torsional constant	
			J	117.82 cm⁴
			Warping constant	
		C_w	0.0 cm⁶	
<u>HSS 76x4</u>				

- Beam length: L =700 cm

Materials properties

Steel G40.21M-350W is used. The following characteristics are used in relation to this material:

- Yield strength $f_y=350$ MPa
- Longitudinal elastic modulus: $E=200000$ MPa
- Shear modulus of rigidity: $G=76923.1$ MPa

Boundary conditions

The boundary conditions are described below:

- Outer:
 - ▶ Support at X = 700 cm (Restraints: TX, TY, TZ, Rx, Ry, Rz)
- Inner: None.

Loading

The column is subjected to the following load combinations and actions:

- ULS: $q = 1.25 \times D + 1.5 \times L$
- LSS: $q = 1 \times D + 1 \times L$

1.1.2.2 Reference results in calculating

Reference solution

From the NBC 2015, the required tension strength for the design is:

ULS
$ Fx = 1.25 \times 20 + 1.5 \times 70$ $ Fx = 130 \text{ kN}$

The Factored tensile resistance T_r of a member subjected to axial tension is computed from the clause 13.2:

$$T_r = \phi \times A_g \times F_y$$

In order to verify the plasticity of the gross section with the factored load computed at ULS:

ULS
$\phi = 0.9$ $T_r = \phi \times A_g \times F_y = 0.9 \times 819 \times 350$ $= 257.985 \text{ kN}$ $T_f = Fx = 130 \text{ kN} < T_r = 257.985 \text{ kN} \text{ O.K}$ Work ratio: $r = \frac{T_f}{T_r} = 50.39\%$

Since the member is under tension forces only the verification of bending resistance and combined forces is unnecessary.

Finite elements modeling

- Linear element: S beam,
- 6 nodes,
- 1 linear element.

1.1.2.3 Results comparison

Result name	Result description	Reference value	AD value	Percent Difference
T_r	Factored tensile resistance	257.985 kN	256.72 kN	0.49%
r	Design ratio	50.39%	50.64%	0.49%

	Unfavorable case	Verification	Work ratio
Tension Compression	n°102	$C_f \leq C_r$ (13.2) 130.00 < 256.72 kN	51%
Combined forces	n°102	$T_f/Tr + M_f/Mr \leq 1$ (13.9.1) 0.506 < 1.000	51%

- Section properties:

Gross cross section

start end

Dimensions(cm)	Outer radius = 3.81 Thickness = 0.40
Cross sections(cm2)	Area = 8.14997 Avy = 1.53552 Avz = 1.53552
Moments of inertia(cm4)	I _t = 107.734 I _y = 53.8672 I _z = 53.8672
Moments of inertia(cm6)	I _w = 0
Moduli of inertia(cm3)	W _{elyinf} = 14.1384 W _{elysup} = 14.1384 W _{elzinf} = 14.1384 W _{elzsup} = 14.1384 W _{ply} = 18.8565 W _{piz} = 18.8565

Material and steel grade

Material:
 Steel grade:



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